



**MAYWOOD RIVERFRONT PARK
ANALYTICAL MODELING**

January 26, 2023

Prepared for:
Los Angeles Neighborhood Land Trust

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Project Number:
184031522

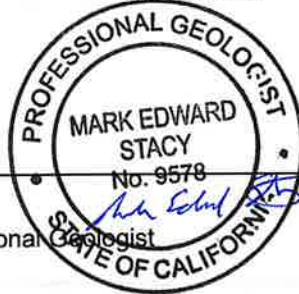
Professional Geologist's Certification

I certify that this analytical model has been prepared by me or under my direct supervision, and that I am a duly registered professional geologist in the State of California responsible for this work.

Signature/Date: _____

Name: Mark Stacy, P.G.

Registration: State of California Professional Geologist
License No. GEO 9578



Exp: 2/29/24

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1 Introduction

This report presents the results of the infiltration facility evaluation performed for Maywood Riverfront Park (Site) in Maywood, part of Los Angeles County, California, as shown on **Figure 1**. Planned renovations for the site, as shown on **Figure 2**, include the installation of a subsurface infiltration facility for the purpose of discharging stormwater into the surficial perched aquifer.

Given the proximity of the nearby Pemaco Superfund Site, as shown on **Figure 3**, and the history of local shallow groundwater contamination, the infiltrating water may have the potential to influence the flow direction and concentration of nearby contaminants. An analytical analysis was performed to assess the potential temporal and spatial impact of the infiltrating water on the perched aquifer and the existing contaminant plume. The primary purpose of this modeling effort was to assess how groundwater mounding from the proposed infiltration facility may or may not move the plume both in terms of direction and distance through different timeframes.



2 Hydrogeology

A saturated zone of permeable, fine to medium grained sand with silt, referred to as the surficial, or perched, aquifer, is separated from the underlying regional aquifers of the Lakewood and San Pedro Formations by a regionally discontinuous, low-permeability clay (TN & Associates, Inc., 2003). Water infiltrating from the surface collects above this clay layer and flows generally toward the south. Water levels within the perched aquifer are generally 20 to 30 feet (ft) below ground surface (bgs) (Arcadis, 2022). The elevation of the clay layer is variable throughout the site, with several peaks and valleys and an overall slope towards the southwest (TN & Associates, Inc., 2003). On average, depth to the clay layer at the site is 35 feet ft bgs. Regionally, the permeability of the clay is also variable, with pockets of siltier, higher permeability material in some areas.

For this analysis, which must model a constant aquifer thickness, the clay layer was assumed to be impermeable and present throughout the site at a depth of 35 ft bgs. Water infiltrating from the proposed infiltration facility is expected to intercept the perched water table and interact with contaminants already present in the aquifer. Any interaction between the water in the shallow aquifer and the deeper regional aquifers was not modeled.



3 Contaminant Concerns

The Site is comprised of approximately 7.8 acres located on properties previously owned and operated by, among others, Precision Arrow, W.W. Henry, Catellus, Lubrication Oil Services, and Pemaco, as shown in **Figure 3**. Stantec reviewed water quality data and previous reports from these sites in order to identify areas and contaminants of concern (COCs). Most of the contamination within the shallow aquifer is the result of the historic on-site usage and storage of a wide variety of chemicals. This area has since been the focus of several monitoring and remediation efforts, although results indicate that areas of concern remain present within the shallow aquifer. In 2005, a Record of Decision (RoD) identified over 56 COCs that exceed United States Environmental Protection Agency (USEPA) primary drinking water standards at the Site - including chlorinated solvents, metals, semi-volatile organic compounds (SVOCs), non-halogenated volatile organic compounds (NHVOCs) and polycyclic aromatic hydrocarbons (USEPA, 2005).

In 2003, a T N & Associates report (2003) investigated the 42 perched aquifer groundwater wells located at or nearby the Site. In their investigation, the primary plumes of COCs that were identified consisted mainly of chlorinated and non-chlorinated VOCs. Metals at the site were detected in every sampled location and were determined to be part of the background water quality, which is generally poor throughout the perched aquifer. The NHVOCs (1,4-dioxane, acrylonitrile, acetonitrile, and methyl isobutyl ketone) were detected at the Pemaco site, but results were often estimates made below the lab's detection limit or were identified in only one sampling event. The SVOCs (bis(2-Ethylhexyl)phthalate and naphthalene) were detected at seemingly random concentrations across the site and could not be related back to a singular plume or release event(s).

The 2003 report (T N & Associates, Inc, 2003) identified one significant source of non-chlorinated VOCs, including toluene and benzene, located within the eastern portion of the former W. W. Henry property. For chlorinated VOCs, including tetrachloroethene (PCE), trichloroethane (TCE), and vinyl chloride (VC), plumes were identified throughout the Pemaco site and eastern W.W. Henry property with an additional PCE source located near District Boulevard approximately half a block south of Pemaco. VC is mainly an end product of TCE and PCE degradation and is not associated with any individual releases.

Updated monitoring reports from 2022 (Arcadis, 2022) indicate that the plumes of chlorinated VOCs within the perched aquifer have not migrated further into the W.W. Henry. A singular monitoring well, VW-3A, within the eastern portion of the W.W. Henry property tested positive for toluene but other COCs have been degraded and/or have been remediated to below detection limits throughout the remainder of the property.

A summary of our current understanding of contaminant plumes within the perched aquifer is presented in **Figure 4**. Please note that this investigation was not intended as a complete characterization of groundwater quality at the Site but instead was used to identify the general location and extent of COC plumes to aid in the quantification of the modeling results. Additionally, this analysis only considered COCs identified in water samples collected from the perched aquifer. Data from soil vapor or contaminants adhered to soils in the unsaturated material above the perched aquifer were not



Maywood Riverfront Park Analytical Modeling

3 Contaminant Concerns

considered, and neither were groundwater data from the deeper regional aquifers. For a full description of the history of contamination at the site and current conditions, please refer to the reports cited above (T N & Associates, Inc., 2003; Arcadis, 2022).



4 Analytical Monitoring

Currently, contaminants in the aquifer are localized and COC plumes are assumed to be relatively stable and/or receding. However, infiltrating large volumes of water into the aquifer could influence the extent and distribution of contamination and create contaminant issues in new areas. To address these concerns, Stantec created an analytical model of stormwater infiltration for a single storm event at the site and looked at the extent of the resulting groundwater mound. Aquifertest Pro 10.0, made by Waterloo Hydrogeologic, was used for the analysis. Modeling was performed using the Theis solution with Jacob correction (Jacob, 1944), for unconfined aquifers. While typically used for aquifer test and pumping analyses, this approach can also utilize negative pumping rates to simulate effects associated with water injection. The resulting negative drawdowns calculated by the program represent modeled groundwater mounding conditions.

Two potential infiltration facility footprints, representing two potential sizing options, were considered. The smaller of the two potential infiltration facility footprints is 0.28 acres (ac) in size and would be located within the footprint of an existing parking lot on the Site. To account for potentially needing an increased amount of water storage during the peak of the storm event, a larger infiltration facility, with a size of 0.56 ac, was also modeled. This larger facility exceeds the existing parking lot dimensions. The location and extent of the two galleries are shown on **Figures 2 and 3**.

The volume of water to be infiltrated corresponds to a single storm with the intensity of the 85th percentile, 24-hour event, as specified by the Los Angeles County's Low Impact Development Standards (County of Los Angeles, 2014). This amount of stormwater was estimated using Los Angeles County Hydrology Map (County of Los Angeles, 2014) to be 2.28 acre-feet of water. The modeling effort did not evaluate the cumulative impacts of multiple storm events.

Due to the lack of on-site testing specific to the perched aquifer, the hydraulic parameters for this analysis were assumed to be somewhere in-between those identified by the National Resource Conservation Services (NRCS) Web Soil Survey and the minimum requirements set forth by the Los Angeles County's Low Impact Development Standards (County of Los Angeles, 2014). The NRCS cites a vertical, saturated, hydraulic conductivity of 79.8 micrometers per second, which is equivalent to 11.32 inches per hour (in/hr) or 22.64 feet per day (ft/day) (NRCS, 2022). The Low Impact Development Standards require a minimum hydraulic conductivity of 0.925 in/hr or 1.85 ft/day. As discussed in the Hydrogeology section, the average depth of the perched clay layer, 35 ft, was used to determine the bottom of the aquifer. An average depth to water of 25 ft was also used, resulting in a calculated aquifer thickness of 10 ft. The resulting maximum and minimum transmissivities used in the model were 226.4 and 18.5 ft²/day, respectively. A storage coefficient of 0.2 was also used. These values are considered representative of a fine to medium grained sand aquifer with silt (Sterrett, 2007) and are consistent with the materials found at the site (TN & Associates, Inc., 2003).



Maywood Riverfront Park Analytical Modeling

4 Analytical Monitoring

To account for vertical anisotropy within the formation, an anisotropy ratio of 1/3 was applied to the maximum and minimum horizontal hydraulic conductivities above to define the vertical hydraulic conductivity that controls the infiltration rates. For each modeling scenario, water was infiltrated at the specified rates until the total volume of 2.28 acre-feet of water was applied. Infiltration rates and times for each modeling scenario are shown in **Table 1**.

Table 1: Modeled Recharge Area and Infiltration Rates Infiltration Facility Scenarios

Model Run	Infiltration Facility Area (ac.)	Modeled Infiltration Rate (in/hr)	Modeled Infiltration Rate (ft/day)	Volumetric Infiltration Rate (ac-ft/day)	Infiltration Time (days)
Large Footprint 1	0.56	3.78	7.55	4.23	0.54
Large Footprint 2	0.56	0.31	0.62	0.35	6.60
Small Footprint 1	0.28	3.78	7.55	2.11	1.08
Small Footprint 2	0.28	0.31	0.62	0.17	13.20

The infiltration of stormwater into the aquifer results in water level increases below the infiltration facility and the outwards propagation of water, forming a structure referred to as a groundwater mound. In order to determine the impact of the gradual expansion and dissipation of this mound, each model was run for a period of over one year. At the beginning of the modeling period, stormwater was infiltrated at a constant rate for the duration listed in **Table 1**. Once the entire 2.28 ac-ft of water was infiltrated, recharge was shut off and the model was allowed to run for an additional year to observe the transition back toward steady state conditions as the mound dissipated.

Due to the limitations of the modeling program, infiltration was modeled as injection applied at specified well locations. A total of 6 wells were placed at equal distances across the smaller infiltration facility footprint whereas 9 wells were placed at the same distance across the larger footprint, as shown in **Figure 5**. As water was injected, the total infiltration rate was divided evenly across each well in the corresponding footprint.



5 Results

Drawdowns for each modeled scenario are expressed as negative numbers to represent a positive mounding condition above the static water table. The maximum height of the groundwater mound occurred at the end of the storm event in all four model scenarios and was greatest at the center of the infiltration facility. The maximum height of the mound was approximately 21 ft in each scenario.

Figures 6, 7, 8 and 9 show the maximum aerial extent of the infiltration mounds for each of the four modeling scenarios, with respect to the known contaminant plume location. For the purposes of this investigation, the extent of the mound was defined as the location of the -0.1 ft contour since mounding of less than 0.1 ft could not be mapped at the scale of the property boundaries. The time at which the maximum extent was reached for the four modeling scenarios varied between 38 days and 521 days, for the large facility footprint with high infiltration scenario (Large Footprint 1) and the small facility footprint with low infiltration scenario (Small Footprint 2), respectively. The time at which the maximum mound extent was observed is presented in **Table 2**.

A change in groundwater level at the edge of the contaminant plume was modeled in each scenario, with impacts beginning at times between 12 days and 167 days for modeling scenarios Large Footprint 1 and Small Footprint 2, respectively. The maximum groundwater level change at the contaminant plume edge was around 0.3 ft for all four scenarios. The groundwater mound dissipated gradually, with impacts at the contaminated property persisting from anywhere between 299 days to over a year. **Table 2** presents the mounding results for each scenario, including the radius of the maximum mounding extent, change in groundwater level observed at the contaminant plume edge, and the time at which the mound began and later stopped influencing the property.

Table 2: Infiltration Facility Mounding Results

Model Run	Time to Maximum Mound Extent (days)	Radius of Maximum Mound Extent (ft)	Change in Groundwater Level at contaminant plume edge (ft)	Time for Mound Extent to Reach contaminant plume edge (days)	Time for Mound Extent to Dissipate* (days)
Large Footprint 1	38	638	-0.31	12	306
Large Footprint 2	486	638	-0.30	146	>365
Small Footprint 1	42	610	-0.30	13	299
Small Footprint 2	521	~565	-0.25	167	>365

*mound dissipation represents the time at which the mound extent no longer touches the contaminant plume



Maywood Riverfront Park Analytical Modeling

5 Results

Using the groundwater flow equation (Sterrett, 2007), the general impact of the change in groundwater level at the Pemaco site boundary can be quantified.

$$\text{Equation 1: } q = -K \frac{dh}{dl}$$

Where:

q = Darcy velocity {L/T}

K = hydraulic conductivity of perched aquifer {L/T}

dh/dl = groundwater gradient; defined as the change (d) in groundwater head (h) over length (l) {L/L}

Based on measurements and figures presented as part of previous groundwater level mapping of the perched aquifer at the Pemaco site (T N & Associates, Inc, 2003) the groundwater gradient in the northwestern corner, where the groundwater mound first enters the property, was estimated to be 0.019 ft/ft towards the south and southeast of the Site. For this calculation, the maximum hydraulic conductivity at the infiltration facility location, 22.7 ft/day, was used. Using these inputs with Equation 1 yields a Darcy velocity of 0.43 ft/day at steady state. If groundwater levels at the northwest corner of Pemaco, where the mound enters the property, increased by a maximum of 0.31 ft, then the groundwater gradient increases to 0.021 ft/ft. This change causes a corresponding increase in the groundwater velocity to 0.48 ft/day.

This Darcy flux represents a potential for groundwater movement over time in the direction of the groundwater gradient. In the case of modeling scenario Large Footprint 1, the length of time for which the mound intercepted the contaminant plume was 294 days. The increase in groundwater velocity, a difference of 0.05 ft/day from steady state, over this time period could account for an additional 14.7 ft of lateral groundwater movement. The difference in flow direction of the groundwater mound versus steady state conditions may move water in a new direction, towards the east instead of the south. Where contaminants are present within the aquifer, this lateral groundwater movement will also push contaminants in the same direction as the resulting groundwater gradient.



6 Conclusions

Assuming a static water level of 25 ft below ground surface, results indicate that none of the modeled scenarios would cause a vertical mound large enough to impact the ground surface. This confirms that surficial flooding would not occur and that there is an ability to infiltrate the desired amount of water into the underlying formation, which is an important first check for infiltration facility design. The smaller of the two infiltration facility footprints considered here remains the preferred design since it fits within the dimensions of an existing parking lot on-site, and findings of the modeling did not support the need for additional storage.

Based on the modeling, existing flow conditions at the Site will be influenced by the stormwater mound, and current flow dynamics in the aquifer may be altered with additional infiltration. In areas of the perched aquifer without contamination, this change is expected to have little consequence, either on aquifer performance or overall groundwater quality. However, the maximum aerial extent of the infiltration mound does overlap with a known area of contamination in all four model runs. A water level change within the area of overlap has the potential to push contaminants in the direction of groundwater flow. While the overall height of the groundwater level increase near the contaminated area was relatively minor, at less than 0.5 ft, results still indicate that infiltration operations will be capable of impacting the current distribution of contaminants in the aquifer. In particular, the calculated Darcy flux and resulting potential for approximately 14.7 ft of additional contaminant plume movement could cause contamination to enter and contaminate adjacent properties.

The direction of the resulting contaminant movement and likelihood of contaminants spreading off the current property could not be fully evaluated as part of the current modeling due to the unknown interaction between the existing groundwater gradient toward the south and the addition of the groundwater mound pushing water toward the east. The resulting groundwater flow direction is likely to be generally southeast, although the exact bearing is not known.

It is worth noting that some geologic features not considered as part of the analytical modeling effort, such as variable aquifer thickness, permeable windows in the confining clay layer, or peaks and valleys in the clay elevation, could impact results, serving to either enhance or trap and diminish the movement of water and contaminants at the Site. One piece of evidence supporting this idea is the overall stabilization of the contaminant plume that has been identified in previous investigations (T N & Associates, Inc, 2003). The groundwater flow calculation performed above indicates that, given the relatively high hydraulic conductivity and existence of an established groundwater gradient, movement of the contaminant plume should be occurring even under steady state conditions. One reason why this stability may have been observed could be the presence of a clay ridge preventing water and contaminants from flowing south at the Pemaco site. If this is the case, then the increase in groundwater head and Darcy velocity identified above may not lead to the level of contaminant movement that would otherwise be expected. A numerical groundwater modeling effort would be needed to fully explore these elements.



7 References

Arcadis, 2007. The W. W. Henry Company Additional Assessment Report.

County of Los Angeles Public Works Department of Public Works, 2014. Low Impact Development Standards Manual.

Jacob, C.E., 1944. Notes on determining permeability by pumping test under water-table conditions, U.S.

Natural Resource Conservation Survey, 2022. Saturated Hydraulic Conductivity (Ksat)- Los Angeles County, California, Southeastern Part. National Cooperative Soil Survey. Accessed online November 14th, 2022.

Sterrett, R.J., 2007. Groundwater and Wells (Third Edition), Johnson Screens.

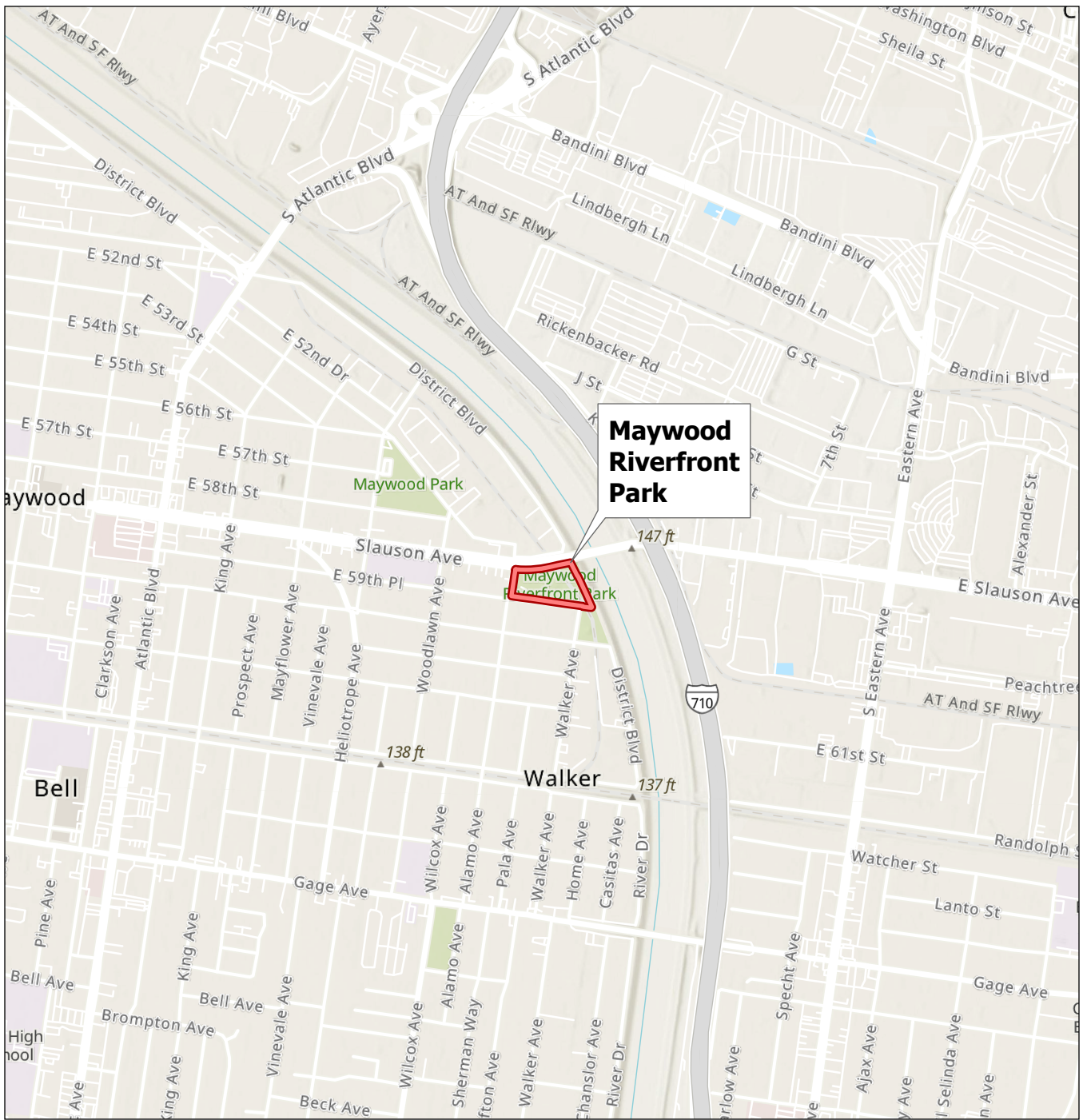
T N & Associates, Inc., 2003. Final Remedial Investigation Report- Pemaco Superfund Site.

United States Environmental Protection Agency, 2005. Record of Decision: Pemaco Superfund Site, Maywood, California. January 13.



FIGURES

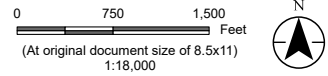




- Notes**
1. Coordinate System: NAD 1927 StatePlane California VI FIPS 0406
 2. Data Sources: Los Angeles County, Stantec, ESRI
 3. Background: ESRI

Legend

Park Boundary



Project Location
T2S, R12W, S19
C. of Maywood, Los Angeles Co., Ca

Prepared by FT on 2023-01-11

Client/Project
TreePeople
Task 4A GLAC IRWM Region
Maywood Riverfront Modeling Project

184031522

Figure No.
1

Title
Site Location



Notes
 1. Coordinate System: NAD 1927 StatePlane California VI FIPS 0406
 2. Data Sources: Los Angeles County, Stantec, ESRI
 3. Background: LA County Aerial Imagery (2011) - Map Service

Legend

- Small Modeling Footprint
- Large Modeling Footprint
- Park Boundary

0 25 50 Feet
 (At original document size of 8.5x11)
 1:600



Project Location Prepared by FT on 2023-01-11
 T2S, R12W, S19
 C. of Maywood, Los Angeles Co., Ca

Client/Project 184031522
 TreePeople

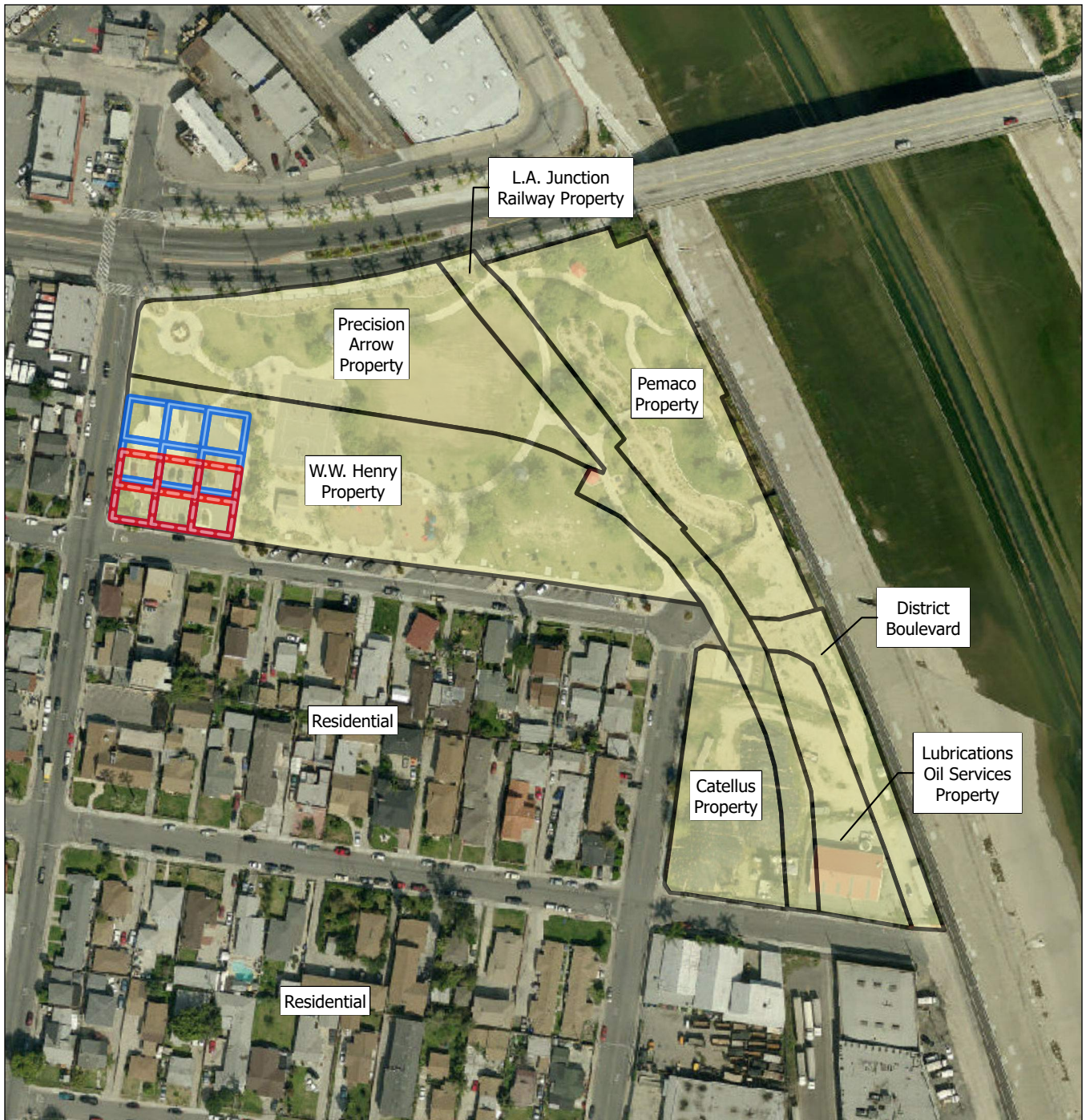
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 Maywood Riverfront Modeling Project

Figure No.

2

Title

Site Detail



Notes
 1. Coordinate System: NAD 1927 StatePlane California VI FIPS 0406
 2. Data Sources: TN and Associates, Inc. (2002)
 3. Background: LA County Aerial Imagery (2011 - Map Service)

Legend

- Former Ownership as of 2002
- Small Modeling Footprint
- Large Modeling Footprint

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 (At original document size of 8.5x11)
 1:2,160



Project Location
 T2S, R12W, S19
 C. of Maywood, Los Angeles Co., Ca

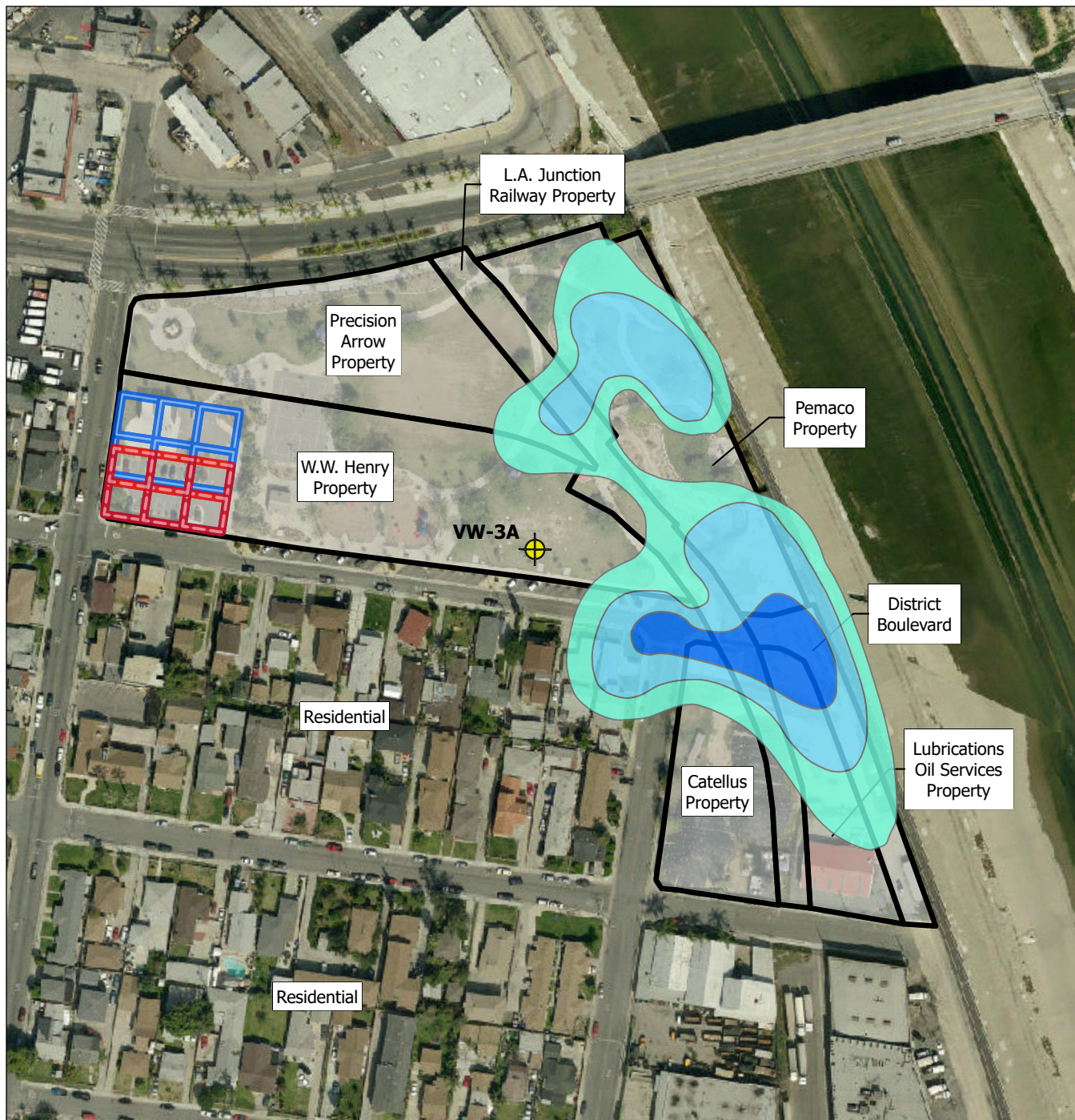
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Figure No.
 3

Title
 Former Site Property



Notes
1. Coordinate System: NAD 1927 StatePlane California VI FIPS 0406
2. Data Sources: TN and Associates, Inc. (2002)
3. Background: LA County Aerial Imagery (2011 - Map Service)

Legend

- DPE Well
- Former Ownership as of 2002
- Small Modeling Footprint
- Large Modeling Footprint
- TCE Plume Concentration in Perched Groundwater Zone, Jan 2002**
 - > 100 ug/L
 - 10-100 ug/L
 - 1-10 ug/L

0 90 180 Feet
(At original document size of 8.5x11)
1:2,160



Project Location T2S, R12W, S19
C. of Maywood, Los Angeles Co., Ca

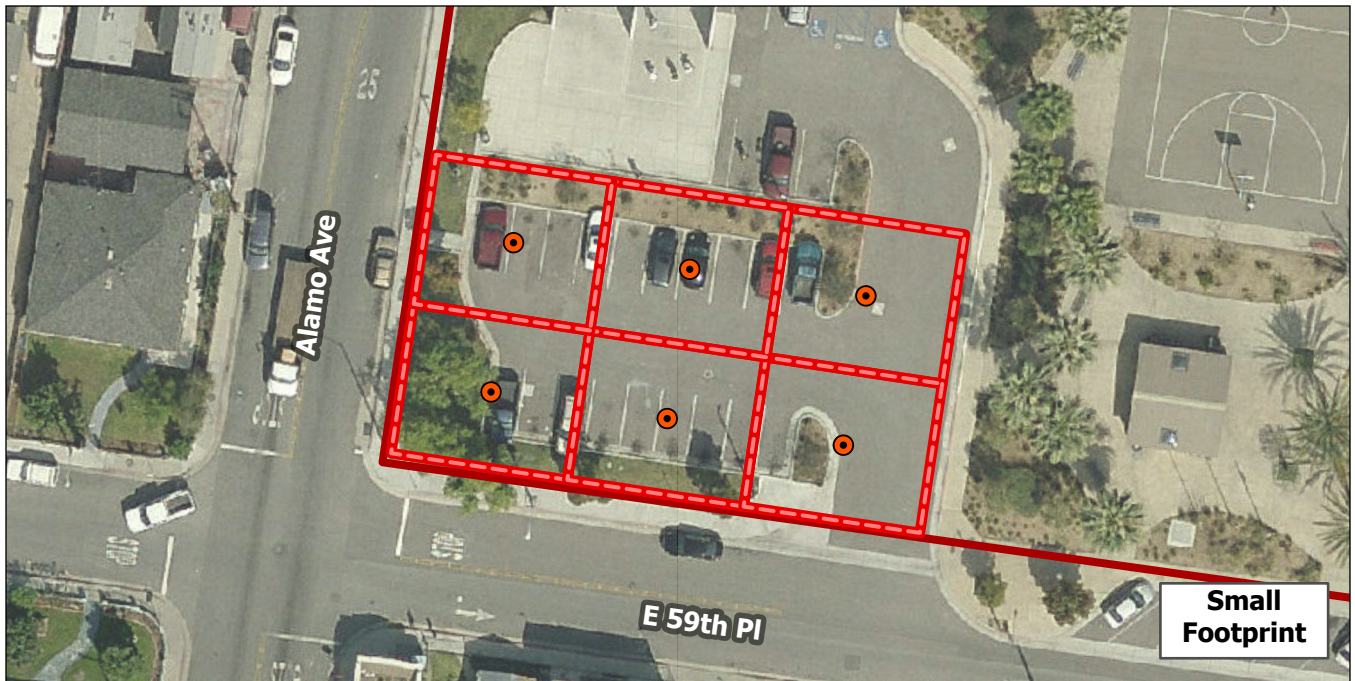
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Figure No. 4

Title
Trichloroethene Contamination Plume



Notes

1. Coordinate System: NAD 1927 StatePlane California VI FIPS 0406
2. Data Sources: Los Angeles County, Stantec, ESRI
3. Background: LA County Aerial Imagery (2011) - Map Service

Legend

- Park Boundary
- Large Modeling Footprint - split into six 2,197 sqft squares
- Hypothetical Injection Wells
- Small Modeling Footprint - split into six 1,824 sqft squares
- Hypothetical Injection Wells

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Project Location
T2S, R12W, S19
C. of Maywood, Los Angeles Co., Ca

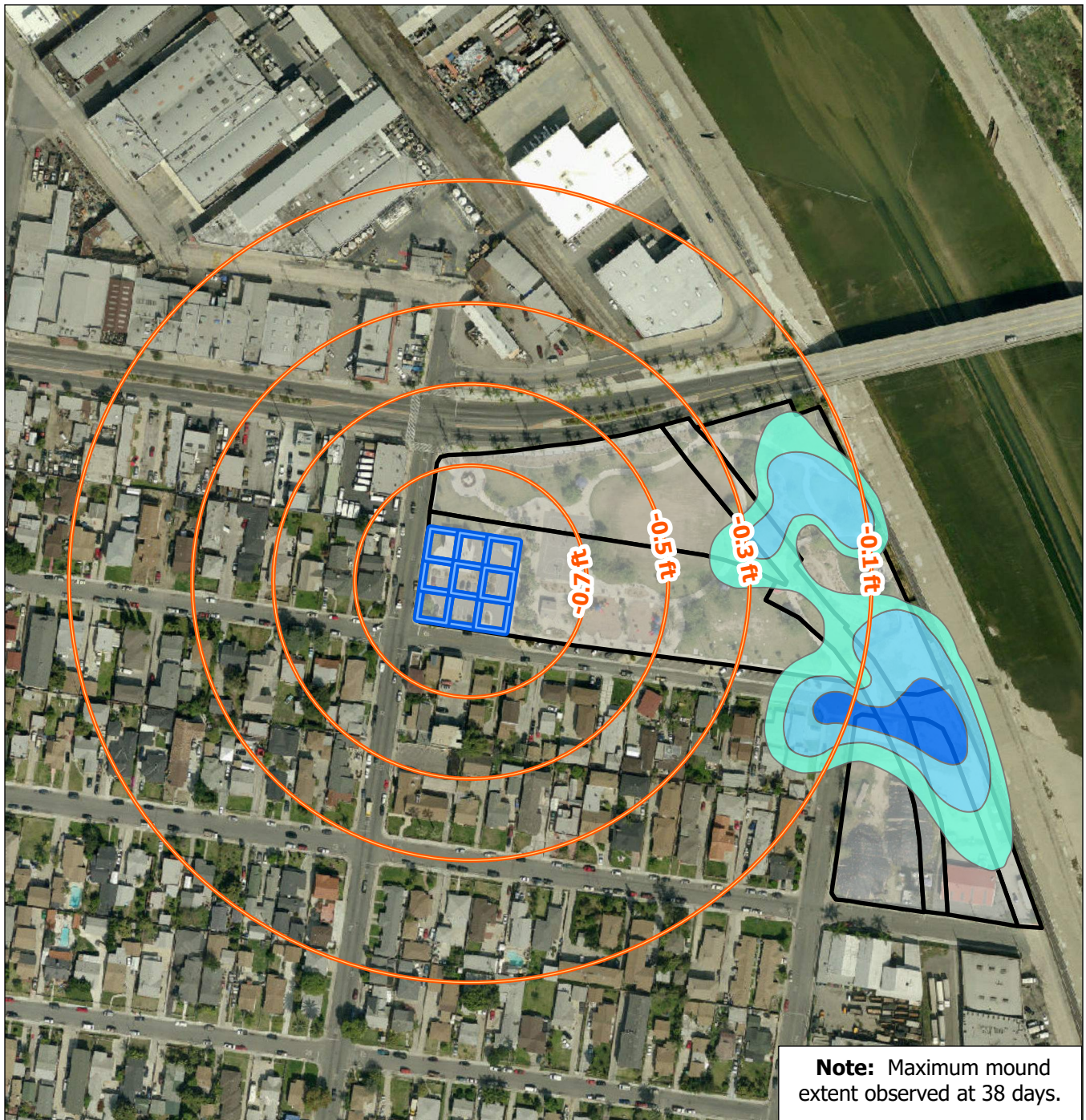
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Figure No.
5

Title
Modeling Setup



Notes
1. Coordinate System: NAD 1927 StatePlane California VI FIPS 0406
2. Data Sources: Los Angeles County, Stantec, ESRI
3. Background: LA County Aerial Imagery (2011) - Map Service

Legend

- Modeled Contours - 0.2 ft contour interval
- Large Modeling Footprint
- Former Ownership as of 2002
- TCE Plume Concentration in Perched Groundwater Zone, Jan 2002
- Concentration
 - >100 ug/L
 - 10-100 ug/L
 - 1-10 ug/L

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(At original document size of 8.5x11)
1:2,880



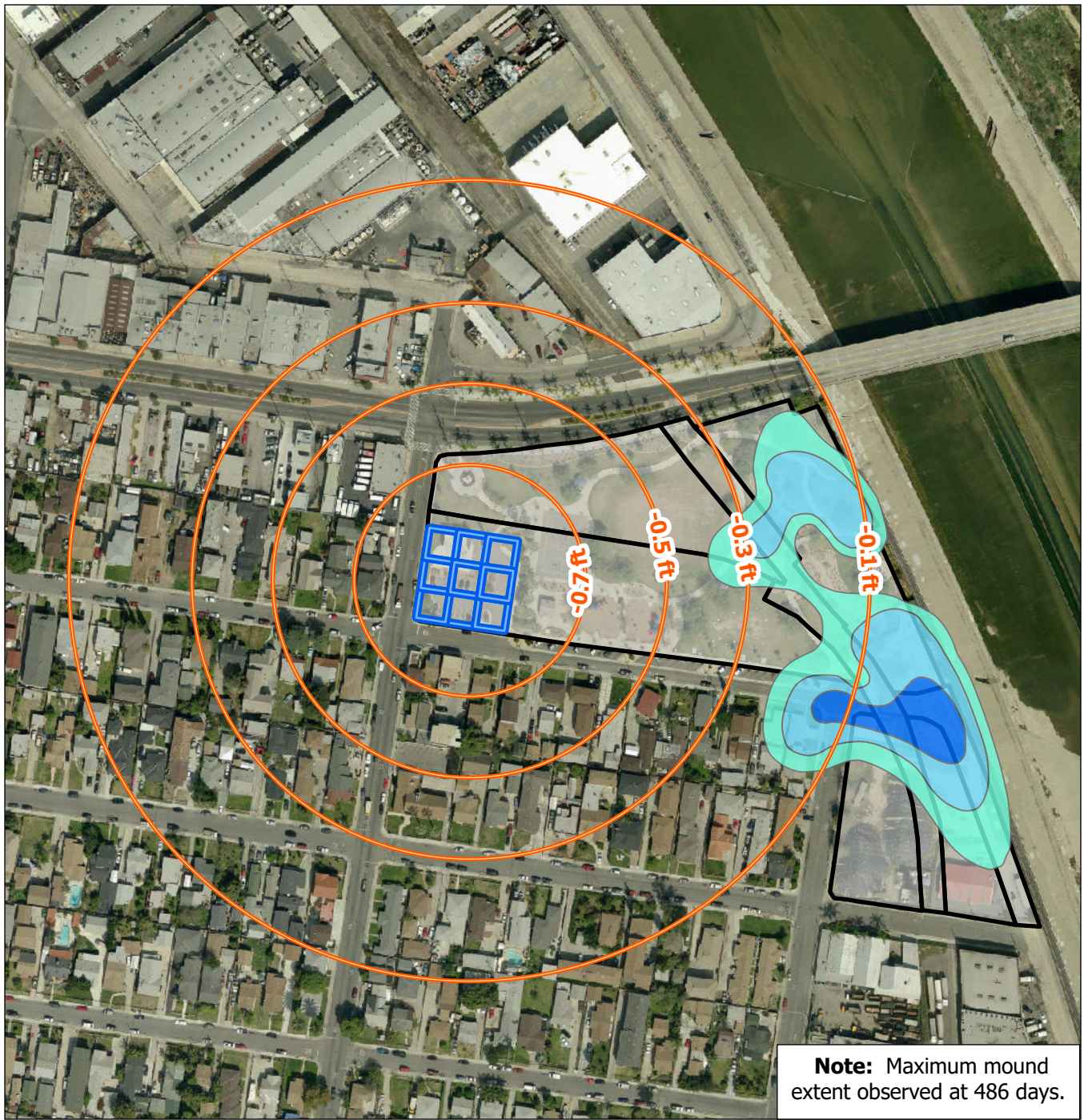
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Maywood Riverfront Modeling Project

Figure No.
6

Title
Large Footprint 1
Large Footprint with High Infiltration



Notes
1. Coordinate System: NAD 1927 StatePlane California VI FIPS 0406
2. Data Sources: Los Angeles County, Stantec, ESRI
3. Background: LA County Aerial Imagery (2011) - Map Service

Legend

Modeled Contours - 0.2 ft contour interval

Large Modeling Footprint

Former Ownership as of 2002

TCE Plume Concentration in Perched Groundwater Zone, Jan 2002

Concentration

>100 ug/L

10-100 ug/L

1-10 ug/L

0 120 240 Feet
(At original document size of 8.5x11)
1:2,880



Project Location
T2S, R12W, S19
C. of Maywood, Los Angeles Co., Ca

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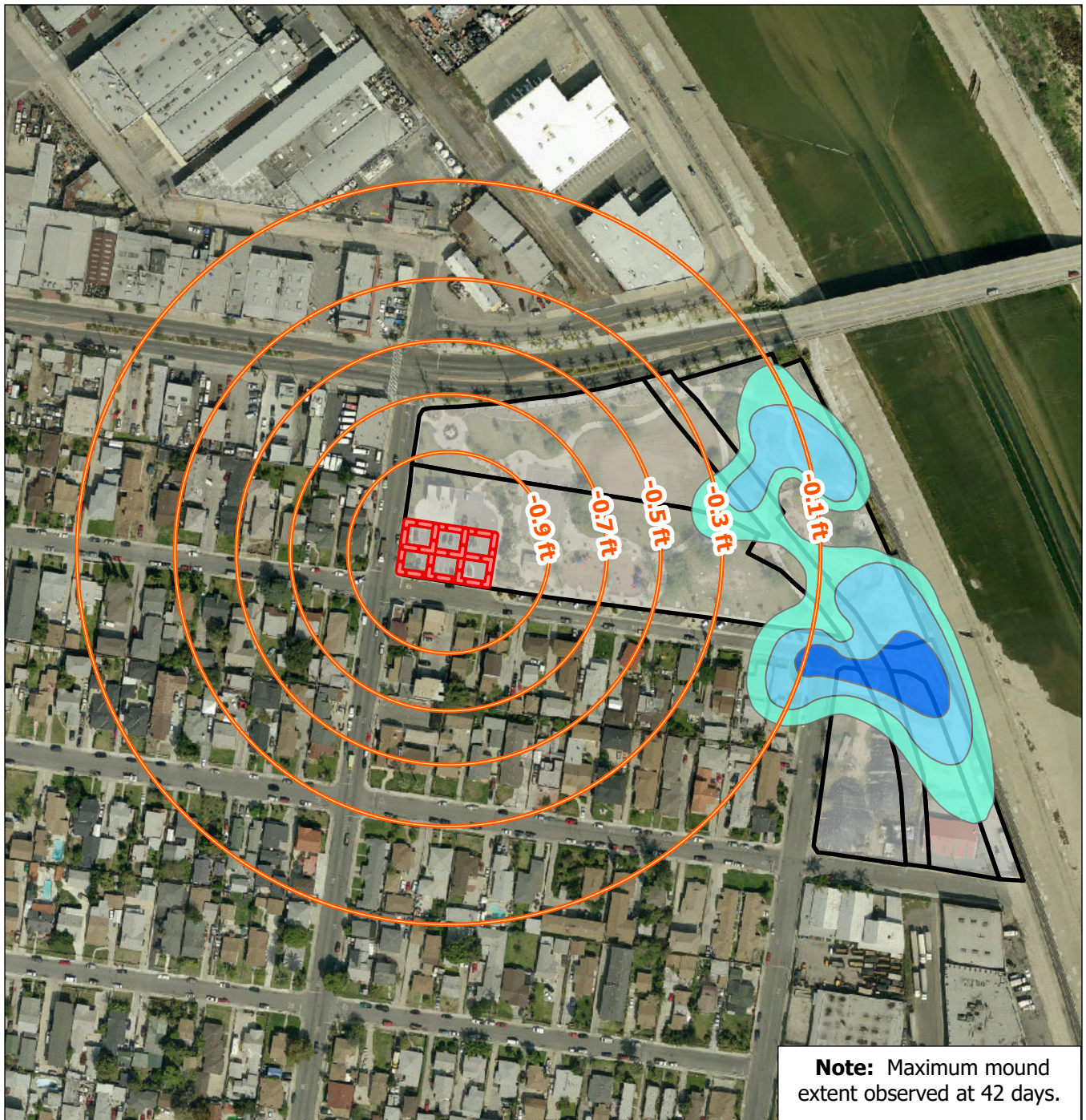
184031522

Figure No.

7

Title
Large Footprint 2
Large Footprint with Low Infiltration

Page 1 of 1



Notes
1. Coordinate System: NAD 1927 StatePlane California VI FIPS 0406
2. Data Sources: Los Angeles County, Stantec, ESRI
3. Background: LA County Aerial Imagery (2011) - Map Service

Legend

- Modeled Contours - 0.2 ft contour interval
- Small Modeling Footprint
- Former Ownership as of 2002
- TCE Plume Concentration in Perched Groundwater Zone, Jan 2002

Concentration

- >100 ug/L
- 10-100 ug/L
- 1-10 ug/L

0 120 240 Feet
(At original document size of 8.5x11)
1:2,880



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Figure No. 8

Title
Small Footprint 1
Small Footprint with High Infiltration



Notes
1. Coordinate System: NAD 1927 StatePlane California VI FIPS 0406
2. Data Sources: Los Angeles County, Stantec, ESRI
3. Background: LA County Aerial Imagery (2011) - Map Service

Legend

- Modeled Contours - 0.2 ft contour interval
 - Small Modeling Footprint
 - Former Ownership as of 2002
- TCE Plume Concentration in Perched Groundwater Zone, Jan 2002

Concentration

- >100 ug/L
- 10-100 ug/L
- 1-10 ug/L

0 120 240 Feet
(At original document size of 8.5x11)
1:2,880



Project Location T2S, R12W, S19
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Maywood Riverfront Modeling Project

Figure No. 9

Title
Small Footprint 2
Small Footprint with Low Infiltration